Ph. D. Thesis Presentation

딥러닝을 이용한 2D 사각 유한요소 개발 Deep Learned 2D Quadrilateral Finite Elements

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Introduction



FEM fields

Deep Learning

Deep learning is a subset of machine learning in artificial intelligence that has networks capable of learning from data







Simple Neural Network



Deep Learning Neural Network



Related Work

- Approaches for applying Deep Learning to numerical analysis
 - J. Sirignano and K. Spiliopoulos (2018), M. Raissi et al(2018-2019)
 - Solving PDE (learning must be performed for each PDE)
 - Z.J. Zhang et al (2015), J. Ling et al (2016), X. Guo et al (2016), J. Tompson et al (2017), O. Hennigh (2017).
 - Flow approximation (not suitable if not trained)
 - Turbulence modeling (general approach, but narrow)
 - L. Liang et al (2018), A. Chamekh et al(2009)
 - FEM surrogate model (not applicable when the domain is changed)
 - M. Amir (2020), T. Kirchdoerfer and M. Oritz (2016), Y.M. Hashash et al (2004)
 - FEM Constitutive models
 - I. Brevis et al (2020, arXiv)
 - FEM weight function parameter tuning using Petrov-Galerkin method (domain dependency problem)
 - A. Oishi and G. Yagawa (2017)
 - Improvement of numerical integration error (general approach, but slight improvement)

Related Work

Model reduction method



- D. Chen (2015), L. Kharevych (2009), Bickel et al (2009)
 - Adapting the constitutive model



- Nesme et al (2009), Barbic and James (2005), Krysl et al. (2001), Shabana (1991)
 - Higher order shape function

Motivation and Objectives

- The most important thing of applying Deep Learning to numerical analysis
 - Generality
- Motivation
 - Developing finite elements using Deep Learning might solve the generality
 - FE feature (suitable to apply Deep Learning)
 - Geometrically bounded shape (1D Line, 2D triangle, 3D hexahedron..)
 - The results of analysis are only determined by physical value inside the element



Comments on Proposal

Prof. Phill-Seung Lee

- Present the applicability and limitation of proposed method. (Title, Closure (topic1), Appendix B-C)
- Consider the solution of the limitation. (Appendix C)
- Prof. Ik Jin. Lee
 - Modify title. (Title)
 - Present the applicability and limitation of proposed method. (Title, Closure (topic1), Appendix B-C)
 - Present the differences in results according to data generation. (Appendix A, D, F)
- Prof. Seunghwa Ryu
 - Check the required training data to increase DOF. (Appendix E)
 - Consider the problem to use the proposed method to 3D element or non-linear problem. (Appendix E, Future work)
 - Consider the problem when the developed element is used to application out of the trained space. (Appendix C)

Comments on Proposal

Prof. In Gwun Jang

- Check whether the proposed method provides sufficient data set for elements with high DOF. (Appendix E)
- Check whether the error can be large in a certain situation. (P22 Error distribution in the test data)
- Suggest another method for linear element. (Topic 2)

Ph.D Yonggyun Yu

- More paper search for data driven FEM. (Related Work)
- Consider cosine annealing, various activation function. (Appendix G)
- When extending the proposed method to 3D elements.
 - Consider various sampling method. (Appendix D)
 - Robust modeling study using bootstrapping and ensemble method. (Future work)
 - Consider various optimization method. (Appendix H)

Research topics

Topic 1. Deep Learned Finite Elements

Overview of DLFE

- Introducing the Deep learned finite elements
 - Objectives
 - Making more suitable strain displacement matrix (**B**) for every shape and material without any rule teaching using deep learning.

$$\boldsymbol{\varepsilon} = \mathbf{B}\mathbf{u} \text{ with } \boldsymbol{\varepsilon} = \begin{bmatrix} \varepsilon_{xx} & \varepsilon_{yy} & \gamma_{xy} \end{bmatrix}^{\mathrm{T}}, \ \mathbf{u} = \begin{bmatrix} u_{1} & \dots & u_{8} & v_{1} & \dots & v_{8} \end{bmatrix}^{\mathrm{T}}.$$



How to make



Overview of DLFE

Introducing the procedure based on 8-node quadrilateral element

Step 1. Data generation



Step 2. Network Training



Step 3. B-matrix generation

Generating elemental K-matrix



Step 4. Elemental K-matrix generation



- Generating strain data according to displacements, geometries, and Poisson's ratios
 - Assumption & Limitation
 - the geometry of 8-node finite elements is limited to a quadrilateral whose mid-side node (nodes 5–8) are placed at the center of the adjacent corner nodes (nodes 1–4).
 - Random geometry
 - The n^{th} normalized random geometry is generated as belows to do efficient learning using geometric similarity.
 - x_1, x_2 are fixed at (0, 0) and (1, 0) respectively.
 - $-x_3$, x_4 are randomly positioned under the constraints that the side length should be less than 1

(0.78, 0.4)

- Severely distorted geometries are excluded
 - 1) interior angle $< 10^{\circ}$ or interior angle $> 170^{\circ}$
 - 2) (maximum side length / minimum side length) > 10



- Generating strain data according to displacements, geometries, and Poisson's ratios
 - Random displacements and Poisson's ratios
 - The *n*th nodal displacements $(\mathbf{u}_i^{(n)})$: randomly generated with a uniform distribution in the range of -0.25 to 0.25
 - The *n*th Poisson's ratios ($\nu^{(n)}$) : randomly applied with a uniform distribution in the range of 0–0.499999999. ($E = 2.0 \times 10^{11}$)
 - Generation of reference data model
 - Reference data model : FEM model has the same geometry as element *n* with a uniform $N \times N$ mesh (N = 50 was used in this study)





Cost function



Data regulation

- Division by zero problem
 - When the strain value ${}^{(i,j)}\hat{\varepsilon}_k^{(n)}$ is close to 0, $C(\theta)$ increases sensitively.
 - Training data containing an absolute value less than 0.005 in the ${}^{(i,j)}\hat{\varepsilon}_k^{(n)}$ were excluded. \rightarrow Almost pure shear, pure stretching data is also excluded.

- Data addition
 - Generating pure shear data on the normalized random geometry.
 - Displacements of *x*-directional shearing data : $\mathbf{u}_{i}^{(n)} = \begin{bmatrix} y_{i}^{(n)} & 0 \end{bmatrix}^{\mathrm{T}}$
 - Displacements of *y*-directional shearing data : $\mathbf{u}_{i}^{(n)} = \begin{bmatrix} 0 & x_{i}^{(n)} \end{bmatrix}^{\mathrm{T}}$
 - The zero-strain components $\rightarrow 0.5\%$ of the maximum strain component in each data.



- Consideration of rigid body motion
 - finite elements should produce zero-strain energy under rigid body motion

$$\sum_{\text{output}}^{(i,j)} \mathbf{B}^{(n)}(\boldsymbol{\theta}) \Delta \mathbf{u} = \mathbf{0} \quad \rightarrow \quad \sum_{l=1}^{8} \sum_{\text{output}}^{(i,j)} \mathbf{b}_{kl}^{(n)}(\boldsymbol{\theta}) = 0 \quad \text{and} \quad \sum_{l=9}^{16} \sum_{\text{output}}^{(i,j)} \mathbf{b}_{kl}^{(n)}(\boldsymbol{\theta}) = 0 \quad \text{for} \quad i, j, k=1, 2, 3$$

rigid body translation)

- Network configuration
 - Framework: Tensorflow



Matrix multiplication











Training configuration

- The number of training data : 300,000 (random : 294,000 + pure shear : 6000).
- The number of test data : 50,000 (random : 48,800 + pure shear : 1200).
- Optimizer : Adam optimizer.
- Weight initialization method : Xavier initializer.
- The number of epochs : 30,000.
- Batch size : 50,000.
- learning rate decaying: $0.01 \rightarrow 0$.



Training result

training data error	test data error
(%)	(%)
1.24	1.67

- Error distribution in the pure random test data
 - The number of test data : 48,800
 - Average of the error: 1.11%
 - Standard deviation of the error : 1.12%



Construction of the stiffness matrix

- Pre-processing of the network input
 - 1) Elemental connectivity modification
 - The side length between \mathbf{x}_1 and \mathbf{x}_2 should be the longest.
 - 2) Translation, rotation, and resizing of the nodal coordinates of the element



- Post-processing of the network output
 - 1) Rotation, and resizing of the output



Construction of the stiffness matrix

- Post-processing of the network output
 - 2) Correction for patch test
 - ${}^{(i,j)}_{\text{DL8}}$ B is approximated by the network. \rightarrow hard to pass the patch tests.
 - Applying B-bar method.

$${}^{(i,j)}_{\text{DL8}}\overline{\mathbf{B}} = {}^{(i,j)}_{\text{DL8}}\mathbf{B} + {}_{\text{DL8}}\mathbf{B'} \text{ with } {}_{\text{DL8}}\mathbf{B'} = \frac{t}{V}\sum_{i=1}^{3}\sum_{j=1}^{3}{}^{(i,j)}w({}^{(i,j)}_{\text{Q8}}\mathbf{B} - {}^{(i,j)}_{\text{DL8}}\mathbf{B}){}^{(i,j)}J$$
$$V : \text{volume of element}$$

Stiffness matrix generation

$${}_{\mathrm{DL8}}\mathbf{K} = t \sum_{i=1}^{3} \sum_{j=1}^{3} {}^{(i,j)} w {}^{(i,j)}_{\mathrm{DL8}} \overline{\mathbf{B}}^{\mathrm{T}} \mathbf{C} {}^{(i,j)}_{\mathrm{DL8}} \overline{\mathbf{B}} {}^{(i,j)} J$$

4-node deep learned finite elements

Applying incompatible element concept



Dividing the matrix to two parts

$${}^{(i,j)}_{\text{DL8}}\mathbf{B}\mathbf{u} = \begin{bmatrix} {}^{(i,j)}\mathbf{B}_{\text{C}} & {}^{(i,j)}\mathbf{B}_{\text{I}} \end{bmatrix} \begin{bmatrix} \mathbf{u}_{\text{C}} \\ \mathbf{u}_{\text{I}} \end{bmatrix}$$

u_C : displacements of compatible nodes
 u_I : displacements of incompatible nodes

Correction for patch test

Stiffness matrix generation

$$\mathbf{K} = \begin{bmatrix} \mathbf{K}_{\mathrm{CC}} & \mathbf{K}_{\mathrm{CI}} \\ \mathbf{K}_{\mathrm{IC}} & \mathbf{K}_{\mathrm{II}} \end{bmatrix} = t \sum_{i=1}^{3} \sum_{j=1}^{3} {}^{(i,j)} w {}^{(i,j)}_{\mathrm{DL4}} \mathbf{B}^{\mathrm{T}} \mathbf{C} {}^{(i,j)}_{\mathrm{DL4}} \mathbf{B} {}^{(i,j)} J \text{ with } {}^{(i,j)}_{\mathrm{DL4}} \mathbf{B} = \begin{bmatrix} {}^{(i,j)} \overline{\mathbf{B}}_{\mathrm{C}} & {}^{(i,j)} \overline{\mathbf{B}}_{\mathrm{I}} \end{bmatrix}$$

$$_{\mathrm{DL4}} \mathbf{K} = \mathbf{K}_{\mathrm{CC}} - \mathbf{K}_{\mathrm{CI}} \left(\mathbf{K}_{\mathrm{II}} \right)^{-1} \mathbf{K}_{\mathrm{IC}}$$

Zero energy mode tests



Table. Eigenvalues corresponding to the 1st~6th modes ($E = 1.5 \times 10^3$, $\upsilon = 0.3$, thickness = 1.0)

Mada	Geometry 1		Geometry 2		Geometry 3		Geometry 4	
Niode	DL8	DL4	DL8	DL4	DL8	DL4	DL8	DL4
1	5.88E-14	4.82E-13	3.32E-13	1.83E-13	1.31E-13	8.31E-13	2.45E-13	7.93E-13
2	3.83E-13	9.43E-13	4.84E-13	7.84E-12	6.68E-13	1.25E-12	7.50E-13	2.86E-12
3	3.36E-03	1.31E-03	2.24E-04	4.87E-03	1.04E-04	5.01E-03	1.44E-03	2.33E-03
4	4.16E+02	1.15E+03	4.33E+02	4.36E+02	1.96E+02	4.69E+02	1.80E+01	7.41E+01
5	4.19E+02	1.15E+03	4.71E+02	5.25E+02	3.87E+02	5.02E+02	1.04E+02	2.73E+02
6	5.07E+02	4.97E+03	4.98E+02	1.14E+03	4.92E+02	1.18E+03	1.23E+02	2.12E+03

Patch tests



- Elements to be compared
 - Linear elements
 - Q4 : standard 4-node quadrilateral element
 - QM6 : 4-node quadrilateral element with incompatible modes
 - P182 : 4-node quadrilateral element in ANSYS 18.0
 - DL4 : Deep learned 4-node quadrilateral element
 - Quadratic elements
 - Q8 : standard 8-node quadrilateral element
 - Q9 : standard 9-node quadrilateral element
 - P183 : 8-node quadrilateral element in ANSYS 18.0
 - DL8 : Deep learned 8-node quadrilateral element
- Strain energy error for convergence curve

$$E_e = \left| \frac{E_{\rm ref} - E_h}{E_{\rm ref}} \right|$$

- $E_{\rm ref}$: strain energy obtained from the reference solution
- E_h : strain energy obtained from the finite element solution

Cook's skew beam problem



Table. Normalized deflections at point A (reference solution: 23.9662).

Mesh	Quadratic elements				Linear elements			
	Q8	Q9	P183	DL8	Q4	QM6	P182	DL4
2×2	0.9479	0.9717	0.9668	0.9868	0.4942	0.8783	0.8783	0.8756
4×4	0.9892	0.9947	0.9900	0.9992	0.7635	0.9604	0.9604	0.9606
8×8	0.9966	0.9983	0.9967	0.9995	0.9213	0.9884	0.9884	0.9883
16×16	0.9987	0.9993	0.9989	0.9996	0.9776	0.9965	0.9965	0.9964

Tapered beam problem

Problem description



 $(E = 3.0 \times 10^2, v = 0.3, \text{ thickness} = 0.1)$

Table. Normalized deflections at point A (reference solution: 0.49679).

Mesh	Quadratic elements				Linear elements			5
	Q8	Q9	P183	DL8	Q4	QM6	P182	DL4
2×6	0.9976	0.9987	0.9982	0.9990	0.9214	1.0006	1.0006	1.0027
4×12	0.9992	0.9995	0.9994	0.9998	0.9781	0.9998	0.9998	0.9999
8×24	0.9997	0.9998	0.9998	0.9999	0.9941	0.9997	0.9997	0.9997
16×48	0.9999	0.9999	0.9999	1.0000	0.9984	0.9998	0.9998	0.9998

Convergence curves



Block problem



Cantilever beam problem



Table. Normalized deflections at point A (reference solution: -3.4694×10⁻³).

Mesh	Quadratic elements				Linear elements			
	Q8	Q9	P183	DL8	Q4	QM6	P182	DL4
Rectangular	0.9861	0.9935	0.9877	0.9862	0.3796	0.9937	0.9937	1.0115
Trapezoidal	0.8984	0.9877	0.9704	0.9940	0.1351	0.2064	0.2064	0.2080
Parallelogram	0.9888	0.9898	0.9997	0.9866	0.1492	0.7932	0.7932	0.7888

Wrench problem *

Problem description



 $(E = 2.0 \times 10^{11}, v = 0.3, \text{ thickness} = 0.01)$

** Wrench problem



0.06

0.12

0.18

х

0.24

0.06

0.12

0.18

х

0.24

Vertical displacement error along the line AB

Computational efficiency

Computational efficiency curves in the Cook's skew beam problem



Computation time : stiffness matrix generation + solving time (Intel(R) Core (TM) i7-2600 CPU @ 3.80 GHz, 12 GB memory, Microsoft Windows 10 64bit, Python environment)

the DL8 element outperforms in the aspect of computational efficiency among the tested elements.

Closure (Topic 1)

- Deep learned finite elements are proposed
 - various new concepts and processes are presented
 - normalized element geometry
 - reference data model
 - pre-processing for the input
 - post-processing for the output
 - A way to make the developed elements better represent rigid body motions and constant strain fields are presented also.
- The performance of the developed elements was evaluated through various numerical examples
 - DL8 showed promising ability in both accuracy and computational efficiency.
- ✤ Limitations of DLFE
 - Geometrical limit (out of train data distribution, see Appendix C)
 - Problem with increasing DOF

Topic 2. Self-Updated Finite Element

Overview of SUFE

- Target element
 - 2D 4-node plane stress elements
- Objectives
 - Generating adaptive B-matrix without any mesh change using displacements to remove shear locking.
- Shear locking
 - In certain cases, the displacements calculated by the finite element method are much smaller than they should be, and it is called locking.
 - Shear locking occurs when elements are subjected to bending.



Overview of SUFE

✤ Concept



Mode based formulation

Original element generation method used in SUFE



Kinematic modes of the linear element

 \clubsuit Generating α according to displacements, geometries, and Poisson's ratios

- Random geometry
 - Normalized geometry is used.
 - Severely distorted geometries are excluded
 - 1) interior angle < 1° or interior angle > 179°
 - 2) (maximum side length / minimum side length) > 100
- Random displacements and Poisson's ratio
 - Normalized by maximum displacement is used.
 - 5 independent displacements is extracted using 3 rigid body modes.
- Strain energy optimization to obtain α
 - Optimization algorithm: Nelder-Mead method
 - Initial seed: 30 seeds evenly distributed in the range of 0° to 90°
- Data set

$$\mathbf{D}^{(n)} = \begin{bmatrix} \boldsymbol{\nu}^{(n)} & \mathbf{D}_{x}^{(n)} & \mathbf{D}_{u}^{(n)} & \boldsymbol{\alpha}^{(n)} \end{bmatrix} \text{ with } \mathbf{D}_{x}^{(n)} = \begin{bmatrix} \mathbf{x}_{3}^{(n)T} & \mathbf{x}_{4}^{(n)T} \end{bmatrix},$$

Network input Label
$$\mathbf{D}_{u}^{(n)} = \begin{bmatrix} \overline{u}_{2}^{(n)} & \overline{u}_{3}^{(n)} & \overline{u}_{4}^{(n)} & \overline{\nu}_{3}^{(n)} & \overline{\nu}_{4}^{(n)} \end{bmatrix}$$

Cost function

$$C(\mathbf{\theta}) = \frac{1}{M} \sum_{n=1}^{M} \left| \alpha_o^{(n)}(\mathbf{\theta}) - \alpha^{(n)} \right|^{\frac{1}{2}}$$

$$\mathbf{\theta} : \text{network weights}$$

M : the number of training data

- $\alpha_{o}^{(n)}(\mathbf{\theta})$: network output
- $\alpha^{(n)}$: reference α
- Network configuration



Training configuration

- The number of training data : 3,000,000
- The number of test data : 50,000
- Optimizer : Adam optimizer.
- Weight initialization method : Xavier initializer.
- The number of epochs : 30,000.
- Batch size : 50,000.
- learning rate decaying: $0.01 \rightarrow 0$.

Training result

Cost function value	Cost function
of training data	value of test data
0.40	0.45

- Average of the error: 0.44°
- Standard deviation of the error
 : 2.49°



The stiffness matrix generation

- Pre-processing of the network input
 - 1) Elemental connectivity modification
 - The side length between \mathbf{x}_1 and \mathbf{x}_2 should be the longest.
 - 2) Nodal displacements rotation and normalize $\rightarrow \mathbf{D}_{u} = \begin{bmatrix} \overline{u}_{2} & \overline{u}_{3} & \overline{u}_{4} & \overline{v}_{3} & \overline{v}_{4} \end{bmatrix}$
 - 3) Translation, rotation, and resizing of the nodal coordinates of the element



- Post-processing of the network output
 - Output modification

 $\alpha = \alpha_o + \beta$

- the stiffness matrix generation
 - $\mathbf{B}_m, \mathbf{K}_m$ generation

The stiffness matrix updating procedures

Iteration procedures



Zero energy mode tests



Table. Eigenvalues corresponding to the 1st~6th modes ($E = 1.5 \times 10^3$, $\nu = 0.3$, thickness = 1.0)

Mode	Geometry 1	Geometry 2	Geometry 3	Geometry 4
1	-2.76E-14	5.58E-14	2.37E-14	1.06E-12
2	-6.12E-14	1.48E-13	-4.25E-14	-3.66E-12
3	-8.60E-14	-1.00E-13	7.37E-13	9.96E-12
4	5.00E+02	4.04E+02	2.61E+02	1.94E+01
5	5.00E+02	8.43E+02	3.18E+02	1.66E+02
6	1.15E+03	1.06E+03	2.62E+03	5.06E+03

Elements to be compared

Symbol	Description
Q4	Standard 4-node quadrilateral element
QM6	4-node quadrilateral element with incompatible modes
P-S	4-node hybrid stress element
SPS	4-node hybrid stress elements with adjustable parameters
<i>B</i> -QE4	4-node assumed strain element (B-bar)
QACM4	4-node incompatible element using QACM-I
NQ6	4-node incompatible hybrid stress element
NQ10	4-node incompatible hybrid stress element
GC-Q6	4-node generalized conforming element
PEAS7	4-node assumed strain element
2D-MITC4/1	4-node assumed strain element based on the MITC method

MacNeal's thin cantilever beam



Table. Normalized vertical displacements at point A in the MacNeal's thin cantilever beam using different meshes, data in bold are the results obtained by the elements proposed in this study, and the number in bracket is the number of iterations.

	_	Load P			Load M	
Elements	Mesh (a)	Mesh (b)	Mesh (c)	Mesh (a)	Mesh (b)	Mesh (c)
Q4	0.093	0.034	0.027	0.093	0.031	0.022
QM6	0.993	0.632	0.051	1.000	0.727	0.045
P-S	0.993	0.798	0.221	1.000	0.852	0.167
PEAS7	0.982	0.795	0.217	-	-	-
QACM4	0.995	0.635	0.052	1.000	0.722	0.046
SU4	0.993(0)	0.994(1)	0.994(1)	1.000(1)	1.000(1)	1.000(1)
Reference	1.000 (t	the value: -	0.1081)	1.000	(the value: -	0.0054)

Cantilever beam divided by two elements for mesh distortion test



(E = 1500, v = 0.25, thickness = 1.0)

Table. Normalized vertical displacements at point A and B in the cantilever beam for mesh distortion test with a distortion parameter e, data in bold are the results obtained by the elements proposed in this study, and the number in bracket is the number of iterations. (Normalized vertical displacements at point A/Normalized vertical displacements at point B)

Elements	е						
	1	2	3	4	4.9		
Q4	0.144/0.141	0.098/0.097	0.083/0.083	0.071/0.072	0.62/0.062		
QM6	0.673/0.627	0.624/0.544	0.657/0.536	0.669/0.512	0.648/0.468		
P-S	0.679/0.629	0.631/0.550	0.672/0.548	0.700/0.630	0.703/0.498		
SPS	1.100/1.000	1.208/1.000	1.332/1.000	1.479/1.000	1.639/0.999		
SU4	1.000/1.000(1)	1.000/1.000(1)	1.000/1.000(1)	1.000/1.000(1)	1.000/1.000(1)		
Exact	1.000/1.000 (the value: 100/100)						

Thin curving beam



((1)
$$h/R=0.03$$
 ($E = 365,010$, $\nu = 0$, thickness = 1) and

(2)
$$h/R=0.006$$
 ($E = 44,027,109$, $v = 0$, thickness = 1)).

Table. The vertical deflection at point A in the thin curving beam, data in bold are the results obtained by the elements proposed in this study, and the number in bracket is the number of iterations.

F 1	Tip deflection			
Elements	<i>h</i> / <i>R</i> =0.03	<i>h/R</i> =0.006		
Q4	0.016	0.001		
QM6	0.650	0.173		
QACM4	0.639	0.026		
SU4	1.005(7)	1.003(4)		
Exact	1.000			

Cantilever beam for rotation dependency test



(E = 100, v = 0.3, thickness = 1.0)

Table. The displacements at point A according to rotation angle in the cantilever beam for rotation	dependency test
using the SUFE, and the number in bracket at the last column is the number of iterations.	

	Tip deflection								
Rotation angle	<i>u</i> _A	$v_{\rm A}$	$\sqrt{u_{\rm A}^2 + v_{\rm A}^2}$	Normalized					
0°	2.4000E-02	4.8000E-02	0.05367	0.9938(1)					
10 °	1.5306E-02	5.1397E-02	0.05363	0.9931(4)					
20°	6.1526E-03	5.3264E-02	0.05362	0.9929(5)					
30°	3.1876E-03	5.3519E-02	0.05361	0.9928(6)					
40°	1.2456E-02	5.2180E-02	0.05365	0.9934(6)					
50°	2.1325E-02	4.9222E-02	0.05364	0.9934(8)					
60°	2.9540E-02	4.4766E-02	0.05363	0.9932(6)					
70 °	3.6846E-02	3.8950E-02	0.05362	0.9929(5)					
80°	4.3173E-02	-3.1982E-02	0.05373	0.9950(3)					
90°	4.8000E-02	2.4000E-02	0.05367	0.9938(1)					
Ref. solution	-	_	0.054	1.000					

Cook's skew beam problem



Table. The tip deflection (v_A) at point A according to mesh densities in Cook's skew beam problem, data in bold are the results obtained by the elements proposed in this study, and the number in bracket is the number of iterations.

Flaments	$v_{\rm A}$					Normalized			
Liements	2×2	4×4	8×8	16×16	2×2	4×4	8×8	16×16	
Q4	11.85	18.30	22.08	23.43	0.492	0.763	0.921	0.978	
QM6	21.05	23.02	23.69	23.88	0.878	0.961	0.989	0.996	
P-S	21.13	23.02	23.69	23.88	0.882	0.961	0.989	0.996	
B -QE4	21.35	23.04	-	23.88	0.891	0.961	-	0.996	
QACM4	20.74	22.99	23.69	-	0.865	0.959	0.989	-	
GC-Q6	27.61	24.31	23.99	-	1.152	1.014	1.001	-	
2D-MITC4/1	19.61	22.64	23.59	23.86	0.818	0.945	0.984	0.995	
SUA	23.80	23.93	23.96	23.95	0.993	0.998	1.000	0.999	
504	(118)	(70)	(6)	(4)	(118)	(70)	(6)	(4)	
Ref. solution	23.9652				1.000				



Closure (Topic 2)

- Self-updated finite elements are proposed
 - New concepts and formulation are presented
 - Stiffness matrix updating method using displacements
 - Mode based element formulation
- The performance of the developed elements was evaluated through various numerical examples
 - SU4 showed promising ability in accuracy.
- ✤ Limitations of SUFE
 - Geometrical limit (out of train data distribution)
 - Iteration time

Conclusions & Future works

Conclusions

- These studies have great implications by showing that artificial intelligence can be used for finite element development.
- The proposed methods are not limited to the elements applied in this studies, and can be extended to various finite elements, including 3D solid, beam, and etc.

Future works

- The expansion of the research to other finite element.
 - Cost function design optimization
 - Network design optimization
 - Robust modeling research
- ✤ Nonlinear analysis.
- ✤ Model uncertainty research.

Thank you

경청해 주셔서 감사합니다

Appendix A

Mesh density of the reference data model **

Problem description



Table. Averaged errors of the trained neural networks according to the mesh density of the reference data model (N).

N	Training data error	Test data error
	(%)	(%)
10	1.57	2.68
30	1.55	1.98
50	1.24	1.67

Linear elements

log h



Appendix B

Convergence behavior of the DL8 element in a curved geometry model



Appendix C

Application out of the trained distribution (extrapolation)

Problem description



Table. Normalized vertical displacements at point A in the cantilever beam for mesh distortion test with a distortion parameter *e*

Elements		e (outside)									
	0	0.5	1	2	3	4	4.9				
Q8	1.000	1.000	0.994	0.894	0.597	0.320	0.197				
Q9	1.000	1.000	1.000	1.000	1.000	1.000	1.000				
P183	0.992	0.992	0.991	0.991	0.992	0.991	0.991				
DL8	1.005	1.009	1.005	1.000	0.991	0.954	0.000				
Exact	1.000 (the value: 100)										



Appendix C

How to overcome the extrapolation limits



Table. Normalized vertical displacements at point A in the cantilever beam for mesh distortion test with a distortion parameter *e*

Elemente		e (inside training area)									
	0	0.5	1	2	3	4	4.9				
Q8	1.000	1.000	0.994	0.894	0.597	0.320	0.197				
Q9	1.000	1.000	1.000	1.000	1.000	1.000	1.000				
P183	0.992	0.992	0.991	0.991	0.992	0.991	0.991				
DL8	1.005	1.009	1.005	1.000	0.991	0.954	0.000				
DL8 +selector	1.005	1.009	1.005	1.000	0.991	0.954	0.995				
Exact	1.000 (the value: 100)										



Appendix D

Effect of data sampling method on network training

Method 1

 $y_3 = \beta_3 \sin(\theta_3)$

 $x_4 = \beta_4 \cos(\theta_4)$

 $y_4 = \beta_4 \sin(\theta_4)$



Figure. Error curves according to the data generation method using the test data generated in (a) Method 1 and (b) Method 3.

Appendix E

- Effect of input degree of freedom on the training
 - In order to increase the input degree of freedom, we randomly rotate the normalized geometry

 $\mathbf{D}_{x}^{(n)} = \begin{bmatrix} \mathbf{x}_{2}^{(n)\mathrm{T}} & \mathbf{x}_{3}^{(n)\mathrm{T}} & \mathbf{x}_{4}^{(n)\mathrm{T}} \end{bmatrix} \longrightarrow \text{network training}$

Table. Averaged errors of training according to the depth and width of the network for each size of the increased input degree of freedom data (training data error (%)/test data error (%)).

Number of	Number of	Number of layers						
weights per layer	Training data	4	5	6	7	8		
378	300,000	4.46/5.96	4.31/6.02	4.20/6.08	4.22/6.21	4.06/6.16		
378	600,000	4.20/4.97	4.04/4.94	4.02/5.01	4.03/5.13	3.88/5.08		
756	300,000	3.04/4.86	3.07/5.04	2.72/4.93	2.90/5.54	2.81/6.00		
756	600,000	2.64/3.64	2.72/3.88	2.85/4.15	2.73/4.06	2.75/4.15		
1512	300,000	2.53/5.97	2.31/4.82	2.21/7.51	2.26/4.85	2.35/5.06		
1512	600,000	2.25/3.51	2.32/3.74	2.02/3.59	2.15/4.12	2.17/3.66		

Appendix F

Effect of network structure and data size

• The number of epochs : 10,000

Table. Averaged errors of training according to the depth and width of the network in Fig. 4.4 for each size of training data (training data error (%)/test data error (%)).

Number of	Number of	Number of layers							
weights per layer	Number of	(network depth)							
(network width)	Training data	2	4	6	8	10			
189	10,000	11.70/22.19	4.49/11.23	3.17/10.32	3.15/10.22	3.16/11.14			
189	50,000	11.55/14.02	3.65/5.69	2.98/5.11	2.82/5.11	2.71/4.96			
189	100,000	11.74/13.20	3.38/4.48	2.80/4.16	2.66/3.97	2.67/4.10			
378	10,000	11.08/20.77	3.77/10.55	2.67/9.73	2.44/9.37	2.50/11.30			
378	50,000	10.28/12.86	2.57/4.63	1.89/4.04	1.97/4.22	1.97/4.39			
378	100,000	9.99/11.51	2.52/3.81	2.05/3.32	1.78/3.13	1.70/3.08			
756	10,000	9.54/19.58	2.60/9.06	2.14/8.83	3.04/9.61	2.17/8.46			
756	50,000	8.96/11.65	1.92/3.93	1.83/3.95	1.46/3.64	1.67/3.82			
756	100,000	9.04/10.47	1.86/3.08	1.57/2.83	1.40/2.63	1.50/2.86			

Appendix G

Effect of network hyper parameter

- The number of training data : 50,000
- The number of test data : 10,000
- The number of epochs : 10,000



Table. Averaged errors of training according to the learning rate, batch size, activation function type (training data error (%)/test data error (%)).

True of	Datak	The learning rate							
activation function	size	0.001	0.005	0.01	0.05	0.01→0 linear	0.01→0 cosine		
ELU	10,000	4.27/6.39	4.23/6.29	4.32/6.19	7.29/10.52	2.43/4.52	2.35/4.33		
ELU	25,000	5.60/7.51	4.63/6.57	4.42/6.21	7.40/10.37	2.20/4.30	2.24/4.44		
ELU	50,000	9.68/11.72	7.50/9.41	8.38/10.67	10.76/12.79	2.55/4.81	2.62/4.82		
Sigmoid	10,000	7.86/9.79	10.90/13.40	14.51/17.82	30.42/37.42	2.07/3.96	2.53/4.90		
Sigmoid	25,000	9.47/11.43	8.87/11.25	13.06/15.27	22.17/26.39	2.41/4.35	2.68/4.76		
Sigmoid	50,000	16.94/18.80	12.03/13.59	16.50/20.07	24.41/30.25	3.33/5.19	3.39/5.32		
tanh	10,000	6.66/8.97	7.37/10.00	8.55/11.82	12.38/19.19	2.20/4.53	2.70/5.01		
tanh	25,000	8.00/10.63	7.41/8.89	8.13/10.21	13.13/16.06	1.97/4.21	2.87/5.46		
tanh	50,000	13.26/15.70	9.84/12.45	11.02/13.28	20.43/24.50	2.60/4.90	2.68/5.08		
Leaky ReLU	10,000	4.40/6.83	6.16/8.24	4.91/7.25	6.91/9.55	2.50/4.95	2.81/5.36		
Leaky ReLU	25,000	6.35/9.47	4.08/6.97	4.01/6.45	7.27/10.24	2.18/5.19	2.40/5.27		
Leaky ReLU	50,000	9.57/13.34	6.73/9.10	6.76/8.79	9.23/12.33	2.37/5.77	2.84/6.00		

Appendix H

Effect of optimizer

- The number of training data : 50,000
- The number of test data : 10,000
- The number of epochs : 10,000

Table.	Averaged error	s of training acc	cording to th	e optimizer	(training da	ata error (%)/test data	error (%)).
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Tuno of	Datah	The learning rate								
optimizer	size	0.001	0.005	0.01	0.05	0.01→0 linear	0.01→0 cosine			
SGD	10,000	89.35/95.12	50.74/54.37	34.74/37.67	13.87/16.72	49.03/52.63	49.40/53.01			
SGD	25,000	116.3/123.6	76.85/81.66	59.29/62.98	24.56/27.12	71.72/76.04	71.84/76.22			
SGD	50,000	127.7/134.7	94.81/100.6	78.46/82.64	38.06/41.14	88.93/93.96	89.82/94.78			
Adagrad	10,000	85.41/92.67	46.64/50.79	28.60/31.65	9.29/11.33	41.02/44.46	42.12/45.78			
Adagrad	25,000	99.44/106.6	64.29/68.68	45.70/49.20	15.82/18.31	57.77/61.76	59.26/63.11			
Adagrad	50,000	114.4/122.1	78.12/83.10	60.97/64.78	25.36/27.79	71.30/75.63	72.42/76.69			
Adam	10,000	4.27/6.39	4.23/6.29	4.32/6.19	7.29/10.52	2.43/4.52	2.35/4.33			
Adam	25,000	5.60/7.51	4.63/6.57	4.42/6.21	7.40/10.37	2.20/4.30	2.24/4.44			
Adam	50,000	9.68/11.72	7.50/9.41	8.38/10.67	10.76/12.79	2.55/4.81	2.62/4.82			
Adam+SGD	10,000	4.38/6.26	3.74/5.81	4.92/6.88	8.21/11.34	2.29/4.40	2.52/4.44			
Adam+SGD	25,000	6.42/8.40	5.01/6.87	5.41/7.31	8.35/11.03	2.03/4.31	2.62/4.75			
Adam+SGD	50,000	11.08/13.28	7.04/8.99	8.58/10.79	10.10/12.71	2.96/5.21	3.23/5.47			